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GEOTECHNICAL SHALLOW SOILS ASSESSMENT REPORT

Project Details

PROJECT STREET ADDRESS	29A Kennedy Road, Fairfield, Dunedin
LEGAL ADDRESS	Lot 2 DP 565372
CLIENT	Trident Homes Dunedin
PROJECT LOCATION	
REVISION NUMBER	0
ISSUED DATE	25/09/2026
RDA JOB NUMBER	54367
RDA JOB NAME	Trident Kennedy Geotech

Prepared by:

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SUMMARY

This report provides the results of a geotechnical shallow soil assessment undertaken for 29A Kennedy Road, Fairfield, Dunedin by RDA Consulting. The investigation was undertaken to provide geotechnical parameters and foundation recommendations for the proposed dwelling development. This report is intended to assist detailed design and support consent applications and is not a detailed design report.

The assessment results represent the general subgrade conditions identified at individual investigation points. Subsoil conditions may vary between these points, and appropriate contingencies should be considered during design and construction. If conditions differ from those described in this report, additional inspection and advice should be sought.

The results of the investigation are summarised below.

Site Description	Address: 29A Kennedy Road, Fairfield, Dunedin Proposed Works: Residential dwelling Land Topography: Moderately sloping Soil Profile: 0.4 m of topsoil, overlying clayey SILT to 2.0 metres below ground level (mbgl) Fill encountered in HA3 to greater than 0.7 mbgl Groundwater Depth: Not encountered
Geotechnical Hazards	No active geotechnical hazards were noted during the desktop or site assessment, including negligible liquefaction risk potential.
Depth To "Good Ground"	In HA1, HA2, HA4 and HA5, "good ground" as defined by NZS 3604:2011 was encountered between 0.30 mbgl and 0.40 mbgl, directly beneath the topsoil. At HA3, fill was encountered to the borehole termination depth of 0.70 mbgl, and the depth to natural good ground was not established.
Foundation Recommendations	NZS3604:2011 (Slab-on-grade, footings, or piles), Specific Engineering and Design, Waffle Slab (e.g. Ribraft, Maxraft, Superslab).
Bearing Capacity Design Depths	300 kPa GUBC - See depth to "good ground" above.
Expected Minimum Excavation Depths for Compacted Gravel Base (Excluding Pile Designs)	Outside the HA3 fill area, the expected excavation depth for removal of topsoil and unsuitable material is approximately 0.30 mbgl to 0.40 mbgl. In the vicinity of HA3, fill extends to at least 0.70 mbgl and its base was not established; excavation may need to extend deeper until competent natural ground is exposed. Removal of all organic soil (topsoil/peat) and non-engineered fill is required.
Further Required Geotechnical Inspections	A subgrade inspection is required in the vicinity of HA3 to confirm the lateral extent and depth of fill and verify that competent natural ground has been exposed before foundation construction. Elsewhere, additional geotechnical assessment is only required where excavated conditions differ from those described in this report, earthfill depths exceed 600 mm, or as required by specific engineering design.
Further Recommended Geotechnical Inspections	Earthfill testing upon completion of earthworks, before foundation construction.
Other Geotechnical Constraints and Recommendations	No further constraints or recommendations noted during the assessment. Please see the full recommendations and discussion section of this report.

1. INTRODUCTION

RDA Consulting were commissioned by Trident Homes Dunedin, in a signed Short Form Agreement dated 18 September 2026, to undertake a Shallow Soils Investigation and Assessment of the site at 29A Kennedy Road, Fairfield, Dunedin (Lot 2 DP 565372).

RDA Consulting's scope of work was to undertake a shallow soils assessment in accordance with Section 3 of the New Zealand Standards for Timber Framed Buildings (NZS 3604:2011) to determine if the soils meet the definition of "good ground". For soil to meet the definition of "good ground" it must have an Ultimate Bearing Pressure of 300 kilopascals (kPa). Evidence of good ground is where foundations on buildings show no signs of settlement, there is no evidence of landslides or buried services nearby, there is no potential for liquefaction or lateral spread, and there is no organic soil (topsoil), peat, soft clay or uncertified or uncontrolled fill.

No Plans for the proposed dwelling were provided. However, based on the recommendations in Sections 3.3.6 and 3.3.8 of NZS3604:2011 four Scala penetrometer tests and four boreholes would be suitable for a building with a footprint of less than 200 m^2.

Due to the cohesive soil material encountered on site, shear vane testing was conducted in lieu of Scala penetrometer testing. Five hand auger boreholes with shear vane testing at 0.3 m increments were conducted, along with one Scala penetrometer test for correlation.

2. BEARING CAPACITY REQUIREMENTS

In addition to the definition provided above, "good ground" will have a Geotech Ultimate Bearing Capacity (GUBC) of not less than 300 kPa when the number of blows per 100 mm depth of penetration at the test site exceeds:

- Five down to a depth equal to twice the width of widest footing below the underside of the proposed footing.
- Three at greater depths.

Providing the set blow count is relatively uniform, the number of blows per 100 mm may be obtained by averaging the number of blows for depths not exceeding 300 mm.

Furthermore, "good ground" must not have the following:

- Potentially compressible ground such as topsoil, soft soils such as clays which can be moulded easily in the fingers, and uncompacted loose gravel which contains obvious voids.
- Expansive soils being those that have a liquid limit of more than 50% when tested in accordance with NZS 4402 Test 2.2 and a linear shrinkage of more than 15% when tested from the liquid limit in accordance with NZS 4402 Test 2.6.
- Any ground which could foreseeably experience movement of 25 mm or greater for any reason including one, or a combination of land instability, ground creep, subsidence, liquefaction, lateral spread, seasonal swelling and shrinkage, frost heave, changing groundwater level, erosion, dissolution of soil in water and effects of tree roots.

The testing method under this standard utilises the Scala penetrometer (Dynamic Cone Penetrometer), however when this method is used with cohesive soils unduly softer results are typically resultant. A more appropriate method of determining the Bearing Capacity of a cohesive soil is the hand shear vane. This test is conducted at regular intervals with depth of the soil profile and provides the direct Undrained Shear Strength of the soil. It is typically conducted in the same hand auger borehole utilised for the soil profile investigations.

For shear vane testing the dial reading is corrected for the individual spring ratio to the blade used with the dial. This undrained vane shear strength is then multiplied by the standard geotechnical rules of thumb by a value of 6 to provide the Geotechnical Ultimate Bearing Capacity at the test point and depth. The test method is in accordance with the "Guideline for Handheld Shear Vane Test" by the New Zealand Geotechnical Society. The shear vane is calibrated yearly to ensure accurate results are achieved.

To provide a correlation between the shear vane and borehole testing, a minimum of one Scala Penetrometer test is conducted for comparison purposes, and to provide a continuous soil profile.

3. DESKTOP REVIEW

3.1. LOCAL GEOLOGY

The geology of the site is mapped on the GNS 1:250k New Zealand Geology Web Map (https://data.gns.cri.nz/geology) as Late Cretaceous - Paleogene sedimentary rocks, consisting of Late Pleistocene river deposits, consisting of Non-marine quartz pebble conglomerate (locally silica-cemented) sandstone, siltstone, mudstone and coal.

3.2. NATURAL HAZARDS

The relevant natural hazards have been assessed from online information. Any further relevant natural hazards noted during fieldwork and their remediation recommendations are included in the Recommendations and Discussion section of this report. A copy of the liquefaction hazard susceptibility map and other relevant natural hazard maps are attached in Appendix A.

LIQUEFACTION

An online review of the Otago Regional Council (https://maps.orc.govt.nz/OtagoMaps/) natural hazard overlay for liquefaction susceptibility shows the site to be classed as being low to none (A-Domain) with ground predominantly underlain by rock or firm sediments. Lateral spreading is often not addressed by these maps, and a site-specific assessment has been incorporated based on a visual assessment of the site and standard geotechnical assessment methods.

To assess the potential risk for liquefaction and/or lateral spreading, RDA use a combination of methods. These include, but are not limited to, a desktop assessment, previous geotechnical knowledge of the immediate area, and the on-site shallow soil investigation results. The results from the liquefaction and lateral spreading assessment are contained within the recommendations and discussion section.

SEISMIC

There is a seismic risk for the region, which is typically mitigated by a prudent design, the MBIE guidelines and NZS3604. A review of the GNS New Zealand Active Faults database (https://data.gns.cri.nz/af/), shows no active faults nearby, or faults considered to expose the site to more risk, above those from regional seismic sources.

No other natural hazards have been identified.

3.3. LAND CONTAMINATION

An online review of the site for soil contamination has been undertaken via the Land Use Sites register. At the time of this review, the site was not identified on the Hazardous Activities and Industries List (HAIL). However, if the site is not on the register, it does not mean that HAIL activity has not occurred there.

4. FIELDWORK

A site visit by RDA Consulting was completed on 21 September 2026. The investigation undertaken during the visit consisted of:

- A site walkover by a geoprofessional.
- Five hand auger boreholes with shear vane testing at regular depth intervals in advance of the borehole.
- One Scala penetrometer test.

A plan of the testing locations is provided in Appendix A, and the test results and investigation logs are provided in Appendix B.

The site was vacant. HA3 encountered fill beneath the surface organic layer and terminated within the fill at 0.70 mbgl due to refusal on buried brick rubble. The base, composition and compaction of the fill, and the condition of the underlying natural soils, were not established. No fill was encountered in HA1, HA2, HA4 or HA5.

Stumps were seen on the surface.

5. RESULTS

Shear vane testing in HA1, HA2, HA4 and HA5 indicates that the natural subgrade generally has a Geotechnical Ultimate Bearing Capacity greater than 300 kPa and is considered very stiff. No shear vane result or depth to good ground was established at HA3 because the borehole terminated in fill at 0.70 mbgl. The Test Location Plan and attached investigation logs show the test locations, soil profiles and test results.

A summary of the results is listed below:

- The soil materials encountered in the boreholes range from a clayey SILT and fill.
- The depth of topsoil varied from 0.3 metres below ground level (mbgl) to 0.4 mbgl at the test locations.
- HA3 encountered fill beneath the surface organic layer to the hand-auger termination depth of 0.70 mbgl. The base of the fill and the depth to natural good ground were not established, due to refusal on buried rubble.
- No groundwater was encountered in any of the boreholes.
- Shear vane tests were conducted to test the in-situ Undrained Shear Strength of the cohesive material.

RDA would advise caution on the strengths measured as seasonal changes in water content of the onsite soils can possibly reduce the measured shear strengths. In drier soil conditions, generally over the summer months, RDA utilise a Factor of Safety (FOS) of 2.0 for utilising the shear vane results in the upper 1.0 m to account for seasonal softening.

The below tables detail the Scala penetrometer, and the shear vane results.

Table 1. Scala Penetrometer Test Results Table (Use Table 2 for depth to "good ground").

SCALA NUMBER	DEPTH TO 300 kPa GUBC (mbgl)	DEPTH TO 200 kPa GUBC (mbgl)	DEPTH TO TEST COMPLETION (mbgl)	DEPTH TO REFUSAL BOUNCING (mbgl)
1	0.55	0.25	1.30	1.30

Table 2. Hand Auger Borehole and Shear Vane Test Results Table.

HAND AUGER NUMBER	DEPTH OF TOPSOIL/ UNSUITABLE MATERIALS (mbgl)	DEPTH OF FILL (mbgl)	SOIL TYPES ENCOUNTERED	DEPTH OF HAND AUGER (mbgl)	**DEPTH TO 300 kPa GUBC (mbgl) "good ground"**	DEPTH TO GROUNDWATER (mbgl)
1	0.40	NE	Clay/Silt	1.80	**0.40**	NE
2	0.30	NE		2.00	**0.30**	NE
3	>0.70	>0.70	Fill	0.70	**NE**	NE
4	0.30	NE	Clay/Silt	1.70	**0.30**	NE
5	0.30	NE		1.60	**0.30**	NE

Notes: NE = Not encountered. For HA3, the hand auger terminated within fill at 0.70 mbgl; the base of fill and depth to natural good ground were not encountered.

The full results of the testing can be found in the attached logs.

6. RECOMMENDATIONS AND DISCUSSION

Please note that the below recommendations and discussion is based on the findings in the test locations only at the time of our investigations and are used as a general representation on subgrade conditions across the site. It is possible and quite likely, that subsoil conditions could change between or below our investigation points, such as rubbish pits, old drain infills, buried organics and other fill materials not visible at the surface, or areas of softer soils present. Contingencies for these occasions should always be considered during your design and construction stages of the project, with additional inspection and advice as required if conditions vary from those in the reporting.

6.1. NATURAL HAZARD ASSESSMENT

LIQUEFACTION

A liquefaction risk assessment has been completed for the proposed dwelling. RDA conclude that the potential risk of liquefaction from a seismic event is negligible beneath the building footprint, due to the cohesive soils encountered. As the potential liquefaction risk is considered negligible, lateral spreading is not considered a potential hazard.

OTHER NATURAL HAZARDS

An RDA representative investigated other potential natural hazards including flooding, land instability, coastal erosion, coastal inundations, active faults, and Tsunami, and assessed these as per the risk matrix, likelihood and consequence tables in Appendix 1 of the National Policy Statement for Natural Hazards 2025. At the time of the investigation RDA determined that

no other significant active natural hazards are noted that have a greater than low natural hazard risk, either pre or post development at this site.

6.2. BEARING CAPACITY

A Scala penetrometer test was undertaken, however, the shear vane results in Table 2 should be used for design unless foundations extend into non-cohesive soil, such as gravel. When Scala penetrometer results are used within cohesive soils, such as silt and clay, unduly conservative measurements are typically resultant. A shear vane test is considered more appropriate to test the strength in cohesive soil.

The undrained shear strengths obtained from the shear vane tests are shown in the attached borehole logs. In HA1, HA2, HA4 and HA5, a 300 kPa GUBC is generally achieved immediately below the topsoil at depths of 0.30 mbgl to 0.40 mbgl, with a factor of safety of 2.0 applied to the undrained shear strength results. At HA3, fill was encountered to the borehole termination depth of 0.70 mbgl; therefore, the depth to natural good ground was not established. Subject to verification and remediation of the HA3 fill area, the remaining good-ground criteria are satisfied in accordance with NZS 3604:2011.

6.3. SITE PREPARATION

Except when using piles, all rubbish, noxious, and organic matter beneath the proposed building footprint and between the "good ground" and the foundation base shall be removed and replaced with compacted certified fill or 10 MPa mass concrete, as per the NZS3604.

Outside the HA3 fill area, removal of topsoil, organic soil and other unsuitable material is expected to require a general strip of approximately 0.30 mbgl to 0.40 mbgl. In the vicinity of HA3, fill extends to at least 0.70 mbgl and its base was not established. Excavation should continue until competent natural ground is exposed, or an alternative specifically engineered foundation or ground-improvement solution should be adopted.

Tree stumps were noted across the site, and deeper excavation may be required where they are removed.

Any foundations on fill shall have the fill placed and compacted in accordance with NZS4431:2022. It is reasonable to expect that fill foundation soils that are placed in accordance with NZS4431 will also achieve 300 kPa GUBC stresses in accordance with NZS3604 Section 3 testing requirements.

If excavations are undertaken and the removed soils are transported using a public roadway, ensure that a stabilised entrance way is constructed at the entrance and exit of the site to enable erosion and sediment control.

6.4. FOUNDATION RECOMMENDATIONS

The foundations suitable for the site are typical NZS3604 types, or waffle raft style slabs designs, such as, MAXRaft, Firth RibRaft, Allied Superslab, or other proprietary foundation systems, with subgrade remediation for the topsoil encountered. Piles may be utilised for design in accordance with the NZS3604: Section 6.

Settlement is expected to be within limits set by NZS3604 for the above Geotechnical Ultimate Bearing Capacity stresses and development and design in accordance with NZS3604 building loads.

Minor seasonal shrink swell effects from soil expansion are likely to occur but are expected to be within acceptable limits as set by NZS3604. These are in class A to S non-reactive to slightly reactive in accordance with AS2870, however detailing for expansive soils is not considered necessary as the plasticity limits above in the good ground definition are not met and the foundation systems recommended in accordance with NZS3604, or the waffle systems are all designed to accommodate this level of movement.

6.5. FURTHER GEOTECHNICAL ASSESSMENT

For NZS 3604:2011 foundations, no further geotechnical assessment is required in the areas represented by HA1, HA2, HA4 and HA5. The extent and depth of fill around HA3 must be confirmed during foundation excavations, and the exposed subgrade should be reviewed by RDA or another qualified geoprofessional before foundation construction. A geoprofessional should also be contacted if subsoil conditions differ from those outlined in this report, expected earthfill depths exceed 600 mm, fill outside the provisions of NZS 3604:2011 is proposed, or excavated subgrades become softened by weather exposure.

7. HEALTH AND SAFETY

This report is not considered a detailed design report. It is a report for others to determine design elements and construction. Any party utilising this information is responsible for ensuring the safety in design considerations and risk framework are

undertaken as well as health and safety in the workplace of all subsequent practitioners and contractors that are encapsulated under the Health and Safety Act.

8. APPLICABILITY

Findings presented as part of this letter are for the sole use of Trident Homes Dunedin, in accordance with the specific scope and the purposes outlined above. While other parties may find this reporting useful, the findings are not intended for use by other parties and may not contain sufficient information for the purposes of other parties or other uses.

Please note this report is not a detailed design report for consent purposes or construction, if design elements are requested RDA can provide this additional scope of works, and any safety in design elements to be considered.

Our professional services are performed using a degree of care and skill normally exercised, under similar circumstances, by reputable consultants practicing in this field currently. No other warranty, expressed or implied, is made as to the professional advice presented in this report.

9. SITE PHOTOGRAPHY



Photo 1: An overview of the site (looking north).

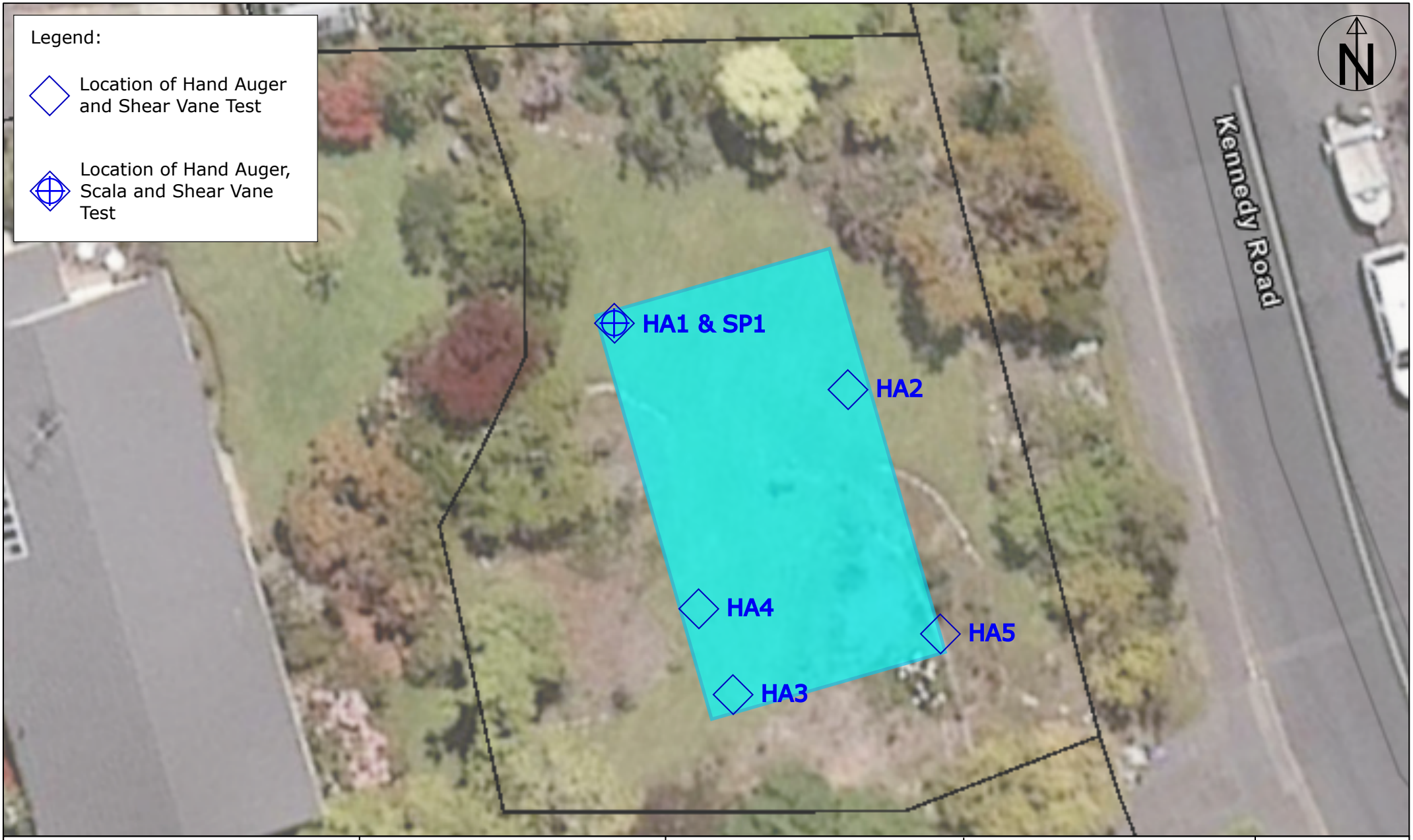
10. APPENDICES

Appendix A – Site Plans

- Investigation Site Plan
- Natural Hazard Map(s)

Appendix B – Investigation Logs

Appendix A – Site Plans



Engineering firm: www.rda.co.nz RDA Consulting GEOTECHNICAL • CIVIL • ENVIRONMENTAL	Project: Trident Kennedy Geotech 29A Kennedy Road, Fairfield, Dunedin	Client: Trident Homes Dunedin	Drawing Title: Test Location Plan Job Number: 54367	Scale: NTS Date:21/09/2026 Drawing No. 1

Natural Hazards Map Print Out



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Liquefaction Awareness Areas Otago

A Domain

World Imagery

Low Resolution 15m Imagery

High Resolution 60cm Imagery

High Resolution 30cm Imagery

Citations

15cm Resolution Metadata

1:758

0 0 0.01 0.02 mi

0 0.01 0.01 0.02 km

Sources: Esri, TomTom, Garmin, FAO, NOAA, USGS, © OpenStreetMap contributors, and the GIS User Community, ORC Natural Hazards. Otago Regional Council., LINZ, DCC, WDC, CDC, QLDC, CODC and ORC, Sources: Esri, Vantor, Airbus DS, USGS, NGA, NASA, CGIAR, N Robinson, NCEAS,

Appendix B – Investigation Logs

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INVESTIGATION LOG

HOLE NO.: HA1 & SP1

CLIENT: Trident Homes Dunedin
PROJECT: Trident Kennedy Geotech

JOB NO.: 54367

SITE LOCATION: 29A Kennedy Road, Fairfield, Dunedin
ELEVATION: Ground Level
DATE: 21/09/2026
CO-ORDINATES: 1398100mE, 4914141mN
LOGGED BY: JT

MATERIAL DESCRIPTION (See Classification & Symbology sheet for details)	DEPTH (m)	LEGEND	SCALA PENETROMETER (Blows / 50 mm)	UNDRAINED SHEAR STRENGTH (kPa)	SENSITIVITY	WATER
Dark brown. Moist; Organic materials, rootlets.	0.2 0.4	TS	300(kPa)GUBC	300(kPa)GUBC VALUES Shear / Residual: 115 / 37	3.1	
Clayey SILT; yellow brown; homogeneous. Stiff to hard; dry to moist; low plasticity.	0.6 0.8 1.0 1.2 1.4 1.6 1.8			UTP -		
End Of Hole: 1.80m	2.0 2.2 2.4					

PHOTO(S)



LEGEND

SP1

REMARKS

End of hole @ 1.8m.
Unable to penetrate further.
No groundwater encountered.

WATER

- Standing Water Level
- Out flow
- In flow

INVESTIGATION TYPE

- [x] Hand Auger
- [ ] Test Pit

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INVESTIGATION LOG

HOLE NO.: HA2

CLIENT: Trident Homes Dunedin
PROJECT: Trident Kennedy Geotech

JOB NO.: 54367

SITE LOCATION: 29A Kennedy Road, Fairfield, Dunedin
ELEVATION: Ground Level
DATE: 21/09/2026
CO-ORDINATES: 1398107mE, 4914140mN
LOGGED BY: JT

MATERIAL DESCRIPTION (See Classification & Symbology sheet for details)	DEPTH (m)	LEGEND	UNDRAINED SHEAR STRENGTH (kPa)	SENSITIVITY	WATER
Dark brown. Moist; Organic materials, rootlets.	0.2	TS	UTP -		
Clayey SILT; yellow brown; homogeneous. Stiff to hard; dry to moist; low plasticity.	0.4 0.6 0.8 1.0 1.2 1.4 1.6 1.8		UTP - UTP - UTP - UTP - UTP -		
End Of Hole: 2.00m	2.0 2.2 2.4				

Calibration Constant: 1.625
300(kPa)GUBC
50 100 150 200
VALUES Shear Residual

PHOTO(S)



REMARKS

Target depth achieved.
No groundwater encountered.

WATER
- Standing Water Level
- Out flow
- In flow

INVESTIGATION TYPE
- [x] Hand Auger
- [ ] Test Pit

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INVESTIGATION LOG

HOLE NO.: HA3

CLIENT: Trident Homes Dunedin
PROJECT: Trident Kennedy Geotech

JOB NO.: 54367

SITE LOCATION: 29A Kennedy Road, Fairfield, Dunedin
ELEVATION: Ground Level
DATE: 21/09/2026
CO-ORDINATES: 1398104mE, 4914129mN
LOGGED BY: JT

MATERIAL DESCRIPTION (See Classification & Symbology sheet for details)	DEPTH (m)	LEGEND	UNDRAINED SHEAR STRENGTH (kPa)	SENSITIVITY	WATER
Dark brown. Moist; Organic materials, rootlets.	0.2	TS	Calibration Constant: 0 300(kPa)GUBC 50 100 150 200 VALUES Shear Residual		
Dark brown. Fill material.	0.4 0.6				
End Of Hole: 0.70m	0.8 1.0 1.2 1.4				

PHOTO(S)



REMARKS

Fill encountered.
No groundwater encountered.

WATER

- Standing Water Level
- Out flow
- In flow

INVESTIGATION TYPE

- [x] Hand Auger
- [ ] Test Pit

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INVESTIGATION LOG

HOLE NO.: HA4

CLIENT: Trident Homes Dunedin
PROJECT: Trident Kennedy Geotech

JOB NO.: 54367

SITE LOCATION: 29A Kennedy Road, Fairfield, Dunedin
ELEVATION: Ground Level
DATE: 21/09/2026
CO-ORDINATES: 1398102mE, 4914132mN
LOGGED BY: JT

MATERIAL DESCRIPTION (See Classification & Symbology sheet for details)	DEPTH (m)	LEGEND	UNDRAINED SHEAR STRENGTH (kPa)	SENSITIVITY	WATER
Dark brown. Moist; Organic materials, rootlets.	0.2	TS	UTP -		
Clayey SILT; yellow brown; homogeneous. Stiff to hard; dry to moist; low plasticity.	0.4 0.6 0.8 1.0 1.2 1.4 1.6		UTP - UTP - UTP - UTP - UTP -		
End Of Hole: 1.70m	1.8 2.0 2.2 2.4				

Calibration Constant: 1.625
300(kPa)GUBC
50 100 150 200
VALUES Shear Residual

PHOTO(S)



REMARKS

End of hole @ 1.7m.
Unable to penetrate further.
No groundwater encountered.

WATER

- Standing Water Level
- Out flow
- In flow

INVESTIGATION TYPE

- [x] Hand Auger
- [ ] Test Pit

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INVESTIGATION LOG

HOLE NO.: HA5

CLIENT: Trident Homes Dunedin
PROJECT: Trident Kennedy Geotech

JOB NO.: 54367

SITE LOCATION: 29A Kennedy Road, Fairfield, Dunedin
ELEVATION: Ground Level
DATE: 21/09/2026
CO-ORDINATES: 1398111mE, 4914131mN
LOGGED BY: JT

MATERIAL DESCRIPTION (See Classification & Symbology sheet for details)	DEPTH (m)	LEGEND	UNDRAINED SHEAR STRENGTH (kPa)	SENSITIVITY	WATER
Dark brown. Moist; Organic materials, rootlets.	0.2	TS	UTP -		
Clayey SILT; yellow brown; homogeneous. Stiff to hard; dry to moist; low plasticity.	0.4 0.6 0.8 1.0 1.2 1.4		UTP - UTP - UTP - UTP - UTP -		
End Of Hole: 1.60m	1.6 1.8 2.0 2.2 2.4		UTP -		

Calibration Constant: 1.625
300(kPa)GUBC
50 100 150 200
VALUES Shear Residual

PHOTO(S)



REMARKS

End of hole @ 1.6m.
Unable to penetrate further.
No groundwater encountered.

WATER

- Standing Water Level
- Out flow
- In flow

INVESTIGATION TYPE

- [x] Hand Auger
- [ ] Test Pit

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